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Kellystown Wind Farm, Drogheda, Co. Louth, Ireland

**Supplementary Ground Investigation
Soakaway Testing**

Report No: 2200-23C Rev0

21st May 2025

*This document has been prepared by Whiteford Geoservices Ltd
on behalf of*

EDF Renewables Ltd



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Kellystown Wind Farm, Co. Louth

[SOILS AND GEOLOGY STUDY – SUPPLEMENTARY
 GROUND INVESTIGATION – SOAKAWAY TESTING]

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Document Control

SIGN OFF		
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1 INTRODUCTION

This report should be read in conjunction with the following reports:-

1. Kellystown Wind Farm - Desktop Study and Walkover Survey for Preliminary Determination of Ground Conditions Report 2200-23A.
2. Kellystown Wind Farm – Preliminary Site Investigation Report 2200-23B.

In April 2025 Whiteford Geoservices Ltd was commissioned by Jennings O’Donovan Ltd to undertake supplementary investigation works at Kellystown Wind Farm near Dunleer, County Louth, Ireland, on behalf of their client EDF Renewables Ltd.

This supplementary investigation was required to determine the permeability of soils to aid soakaway design for the construction of the proposed Kellystown Wind Farm.

Prior to undertaking these supplementary investigation works preliminary investigations had been undertaken for the site to provide a broad understanding of the prevailing ground conditions present.

The ground investigation works, detailed within this report, consist of the following elements.

The scope of these works was as follows: -

Ref	SI Component	Remarks
a	Trial pitting to determine ground conditions for In-situ Soakaway Testing.	<p>A total of 10nr Trial pit exploratory holes to assess ground conditions. Each trial pit was excavated to a maximum depth of 0.54m and 1.40m.</p> <p>These exploratory holes were undertaken at the following locations:</p> <ol style="list-style-type: none"> 1. TP-S1 – carried out for proposed soakaway at S1 2. TP-S2 – carried out for proposed soakaway at S2 3. TP-S3 – carried out for proposed soakaway at S3 4. TP-S4 – carried out for proposed soakaway at S4 5. TP-S5 – carried out for proposed soakaway at S5 6. TP-S5A – second test carried out for proposed soakaway at S5 7. TP-S6 – carried out for proposed soakaway at S6 8. TP-S7 – carried out for proposed soakaway at S7 9. TP-S7A – second test carried out for proposed soakaway at S1 10. TP-S7B – second test carried out for proposed soakaway at S1

Table 1 – Scope of Supplementary Ground Investigation Works

The investigation was performed in accordance with the relevant standards (see References) and data presented within the relevant appendix to this report.

This report presents the factual records of the investigations undertaken.

2 SITE AND GEOLOGY

2.1 The Site

Kellystown Wind Farm is situated approximately 4.5km south of Dunleer, County Louth and straddle undulating agricultural grazing lands between Gallstown Quarry in the north and Piperstown Livery and Equestrian Centre to the south.

Ground surface elevations vary between approximately 91m to 126m above Ordnance Datum (Malin Head).

The land usage does not appear to vary across the number of land holdings which make up the Kellystown Wind Farm development area and consist of agricultural grazing land for cattle.

The closest active quarrying operation to the site, is Gallstown Quarry, immediately bordering the north of the site, which is operated by Kilsaran Concrete Ltd.

2.2 Published Geology

A study was made of available geological information for the area (GSI Online Database). This study indicated that the following natural geology is present across the site of Kellystown Wind Farm.

- Thickly bedded calcareous greywacke (siltstone) which outcrops within the western portion of the site
- Glacial Till (derived from Lower Palaeozoic sandstones and shales)
- Alluvium (present in river valley bottoms)
- Cutover Raised Bog (isolated discrete locations)

2.2.1 Solid Geology

According to the GSI online database, the proposed development area for Kellystown Wind Farm site is immediately underlain by the Clogherhead Formation which consists of, thickly bedded calcareous greywacke.

Also present just outside the development area, northeast of the site boundary, is the Red Mans Cove Formation which consists of, red, green, black mudstone

A series of three faults bisect the proposed development area trending in an approximately north northeast – south southwest direction but will have negligible effect in relation to soakaway design.

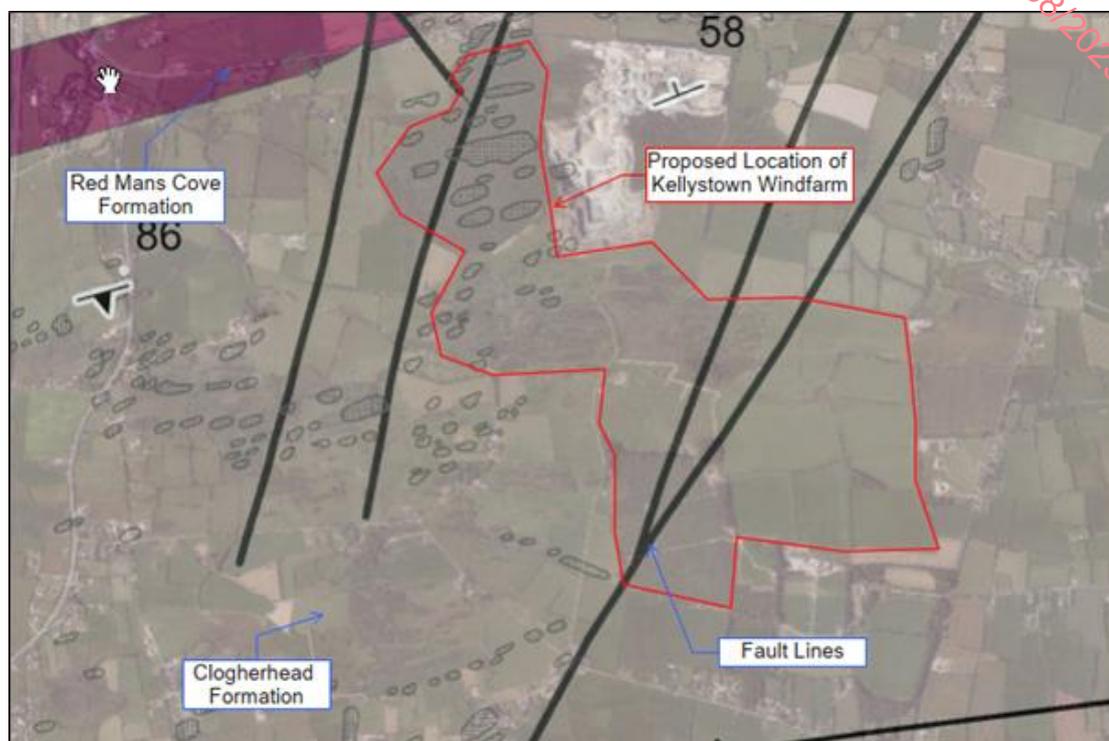


Figure 1 – Bedrock Solid Geology
Reproduced courtesy of GSI Datasets Public Viewer

2.2.2 Superficial Geology

Superficial soils present within the wind farm consist of thin glacial till soils overlying shallow, often outcropping greywacke rock to the north and west. Thicker glacial tills derived from Lower Palaeozoic sandstones are present towards the south and east of the site.

Alluvium is present throughout the Site with some isolated area of peat also indicated within the site, although neither are present at the location of the exploratory holes undertaken as part of this ground investigation campaign.

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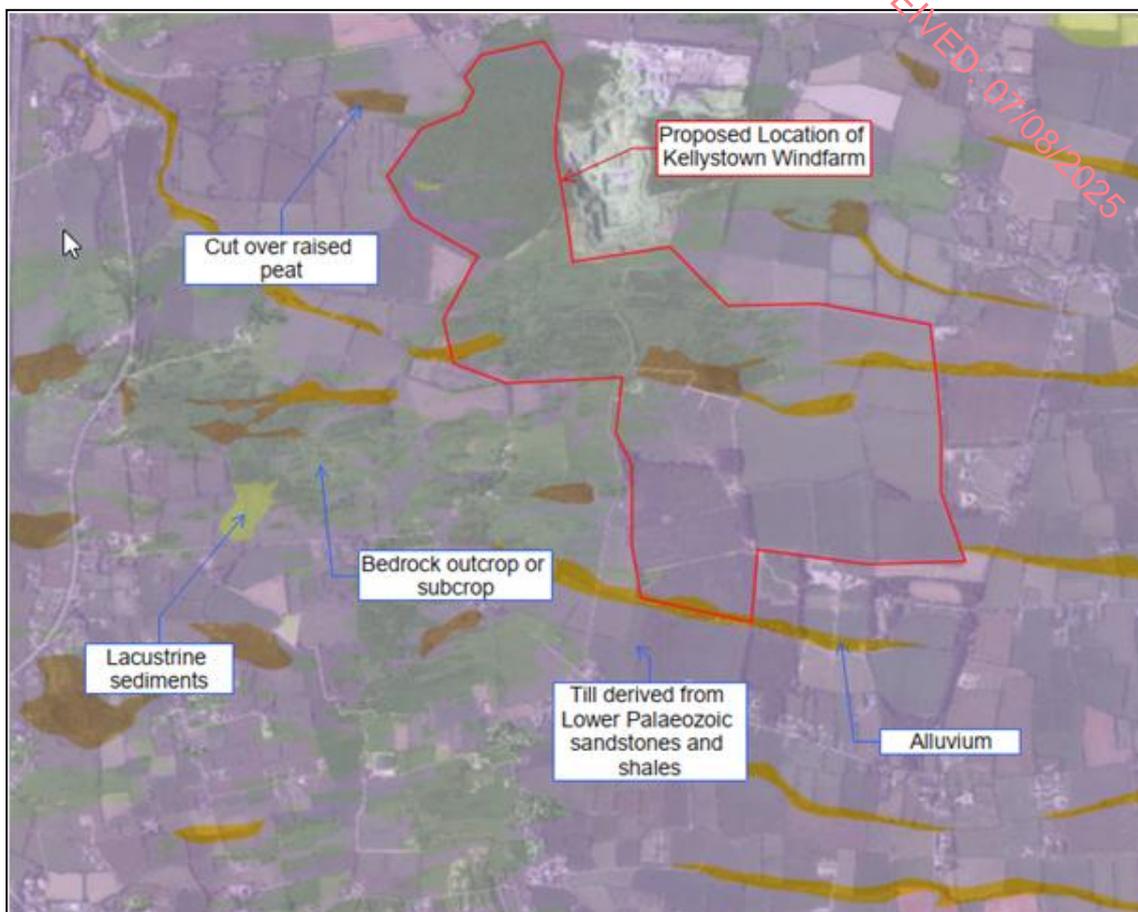


Figure 2 – Superficial Geology
Reproduced courtesy of GSI Datasets Public Viewer

3 FIELDWORK

3.1 General

All fieldwork was carried out in general accordance with BS 5930:2015+A1:2020 and other related standards.

Please refer to Appendix A for the location of all geotechnical investigations undertaken.

3.2 Exploratory Holes

The exploratory holes are detailed within the following table.

METHOD	QUANTITY	MAXIMUM DEPTH (m)	EQUIPMENT
Trial Pit	10 Nr.	1.40	Trial pits were carried out with the use of a 13T Tracked Excavator

Table 3 – Schedule of Exploratory Holes

Refer to Appendix B for engineering logs of trial holes

3.3 In-situ Testing

The in-situ testing works carried out are detailed within the following table.

TYPE	QUANTITY	MAX. DEPTH (M)	EQUIPMENT
Soakaway Permeability Testing (BRE Digest 365)	10 ¹	1.40	1000 litre water bowser

Table 4 – Schedule of In-Situ Tests

Refer to Appendix B for in-situ testing reports

¹ At S1, S2, S3, S4 and S6 one single set of soakaway tests were undertaken. At S5 and S7 multiple tests were carried out; 2 no. and 3 no. respectively. These tests were carried out within different trial holes.

3.4 Topographical Survey

A topographical survey of exploratory hole locations was undertaken post completion of all associated investigation works and is detailed in the table below.

EQUIPMENT	COORDINATE SYSTEM
Leica RTK / GNSS DGPS System	Irish Transverse Mercator (ITM) / Malin Hean (Ordnance Datum)

Table 5 – Topographic Surveying

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4 SUMMARY OF FINDINGS

The following table summarises the findings from the Supplementary Ground Investigation campaign in respect to soil permeability.

TEST REF:	PERMEABILITY (M/S)	OBSERVATIONS
S1	1.16×10^{-5}	Moderately Permeable
S2	6.24×10^{-4}	Moderately to Highly Permeable
S3	3.49×10^{-6}	Moderately Permeable
S4	$<1 \times 10^{-10}$	Poor Permeability – Practically no drop in water level
S5	N/A	Test Stopped – Water Table encountered
S5A	2.78×10^{-5}	Moderately Permeable
S6	4.42×10^{-6}	Moderately Permeable
S7	N/A	Test Stopped – Practically no drop in water level
S7A	N/A	Test Stopped – Practically no drop in water level
S7B	$<1 \times 10^{-10}$	Poor Permeability – Practically no drop in water level

Table 7 - Summary of Findings

5 REFERENCES

BRE Digest 365 (2016): Soakaway Design, Watford, UK, BRE Group.

BS 5930:2015 + A1:2020 Code of practice for ground investigations. British Standards Institution.

BS EN 1997-2: 2007 : Eurocode 7 - Geotechnical design - Part 2 Ground investigation and testing. British Standards Institution.

BS EN ISO 14688-1: 2002 : Geotechnical investigation and testing - Identification and classification of soil - Part 1 Identification and description. British Standards Institution.

BS EN ISO 14689-1: 2003 : Geotechnical investigation and testing - Identification and classification of rock - Part 1 Identification and description. British Standards Institution.

BS EN ISO 22475-1: 2006 : Geotechnical investigation and testing – Sampling methods and groundwater measurements - Part 1 Technical principles for execution. British Standards Institution.

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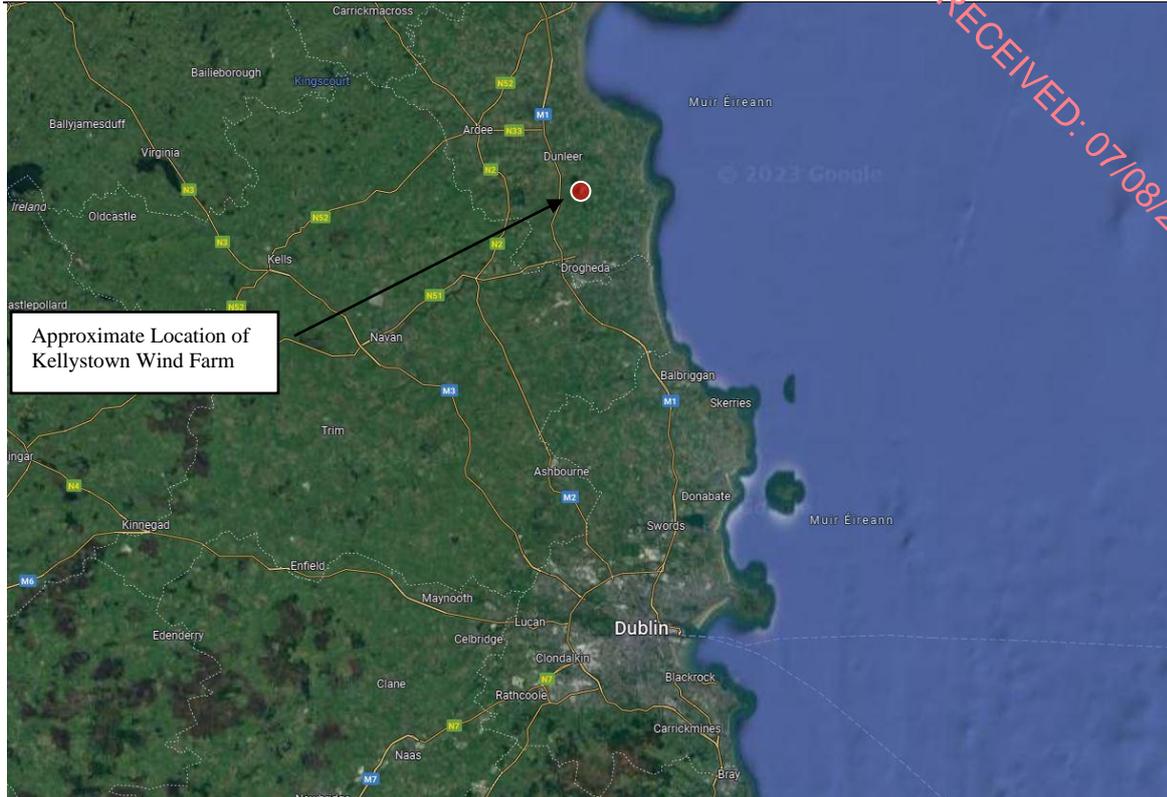
APPENDICES

Appendix A	Drawings
Appendix B	Exploratory Holes and In-situ Test Results
Appendix C	Soakaway Test Specification
Appendix D	Photographic Record

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APPENDIX A DRAWINGS

General Site Location Plan	1 x A4
Site Layout Plan showing position of Exploratory Holes and Insitu Tests 2200/23C – L1	1 x A3



P1 - General Location Plan (Aerial view)
(© Google Maps 2025)

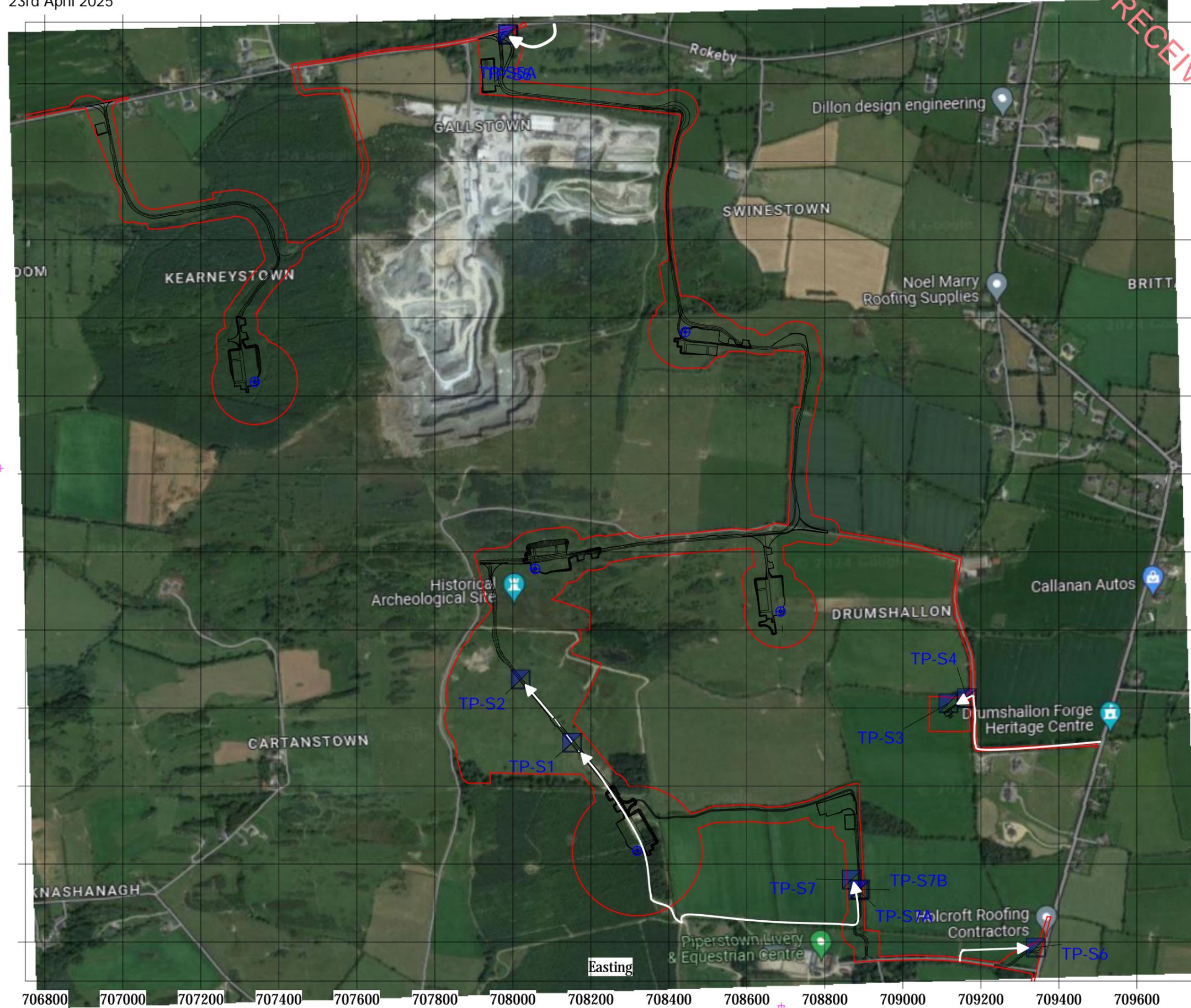


P2 - Local Location Plan – Kellystown Wind Farm (Aerial view)
(© Google Earth 2025)

2200-23C - Kellystown Wind Farm

Supplementary Ground Investigations - Site Layout Plan including Exploratory Hole Locations

23rd April 2025



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- KEY**
- Redline Boundary
 - Wind Turbine Generator
 - TP-S1
 - Proposed Exploratory Trial Hole (2025 Soakaway Testing)

Easting	Northing	ID
708153.5	783112.1	TP-S1
708020.6	783273.4	TP-S2
709116.2	783194.9	TP-S3
709163.6	783227.7	TP-S4
707989.4	784925.5	TP-S5
709341.5	782586.1	TP-S6
708869.8	782761.1	TP-S7



- Notes:**
1. All Positions are relative to ITM
 2. Elevation are relative to Malin Head Datum

Designed by JW	Checked by JW	Approved by JM 21/05/2025	Drawing No. 2200/23C - L1	Date 21/05/2025	Scale 1 : 10,000
Whileford Geoservices Ltd Strald House 2 Main Street, Strald, BALLYCLARE, Co. Antrim, UK BT39 9 NE			Supplementary Ground Investigations As-built Exploratory Hole Plan Kellystown Wind Farm		
				5	A3

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APPENDIX B

IN-SITU TEST RESULTS AND LOGS OF EXPLORATORY HOLES

Soakaway Testing Data Sheets	10 x A4
Trial Pit Logs	10 x A4

Trial Pit Log

Project Name: Kellystown Wind Farm		Client: EDF Renewables		Date: 30/04/2025	
Location:		Contractor:		Co-ords: E708151.76 N783111.38	
Project No. : 2200-23		Crew Name:		Equipment: Hitachi 13T excavator	
Location Number TP-S1	Location Type TP	Level 104.42m AoD	Logged By Jack Rainey	Scale 1:10	Page Number Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
					0.20	104.22		Brown SILT (TOPSOIL)
		0.60	B		0.60	103.82		Brown slightly gravelly clayey SILT with low cobble content.
								End of Borehole at 0.600m

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks
0.90	0.50	No issues.					

Remarks
No groundwater encountered.



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Trial Pit Log

Project Name: Kellystown Wind Farm		Client: EDF Renewables		Date: 30/04/2025	
Location:		Contractor:		Co-ords: E708019.63 N783272.87	
Project No. : 2200-23		Crew Name:		Equipment:	
Location Number TP-S2	Location Type TP	Level 102.08m AoD	Logged By	Scale 1:10	Page Number Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
		0.50	B		0.55	101.53	 Brown very sandy very gravelly SILT with medium cobble content.	
							End of Borehole at 0.555m	

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks
No groundwater encountered.



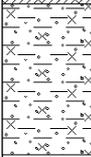
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1

2

Trial Pit Log

Project Name: Kellystown Wind Farm		Client: EDF Renewables		Date: 01/05/2025	
Location:		Contractor:		Co-ords: E709117.05 N783211.97	
Project No. : 2200-23		Crew Name:		Equipment:	
Location Number TP-S3	Location Type TP	Level 89.02m AoD	Logged By	Scale 1:10	Page Number Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
							Brown slightly gravelly SILT (TOPSOIL)	
				0.30	88.72		Brown slightly gravelly silty CLAY.	
		0.50	B					
				0.50	88.52		End of Borehole at 0.500m	

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

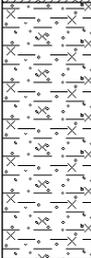
Remarks
No groundwater encountered.



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Trial Pit Log

Project Name: Kellystown Wind Farm		Client: EDF Renewables		Date: 30/04/2025	
Location:		Contractor:		Co-ords: E709163.88 N783225.24	
Project No. : 2200-23		Crew Name:		Equipment:	
Location Number TP-S4	Location Type TP	Level 88.18m AoD	Logged By	Scale 1:10	Page Number Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
					0.20	87.98		Brown SILT (TOPSOIL)
		0.50	B		0.54	87.64		Brown slightly gravelly silty CLAY.
								End of Borehole at 0.540m

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

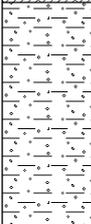
Remarks
No groundwater encountered.



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Trial Pit Log

Project Name: Kellystown Wind Farm		Client: EDF Renewables		Date: 01/05/2025	
Location:		Contractor:		Co-ords: E707989.43 N784924.92	
Project No. : 2200-23		Crew Name:		Equipment:	
Location Number TP-S5	Location Type TP	Level 84.82m AoD	Logged By	Scale 1:10	Page Number Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
					0.20	84.62		Brown SILT (TOPSOIL)
					0.50	84.32		Brown gravelly silty CLAY
					0.60	84.22		Brown clayey silty fine to coarse angular GRAVEL. Highly weathered bedrock (Distinctly / residual soil).
								End of Borehole at 0.600m

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1

2

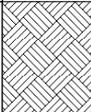
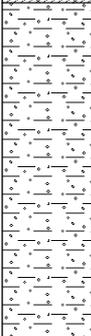
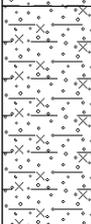
Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks
No groundwater encountered.



Trial Pit Log

Project Name: Kellystown Wind Farm		Client: EDF Renewables		Date: 01/05/2025	
Location:		Contractor:		Co-ords: E707986.78 N784927.85	
Project No. : 2200-23		Crew Name:		Equipment:	
Location Number TP-S5A	Location Type TP	Level 85.01m AoD	Logged By	Scale 1:10	Page Number Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
					0.15	84.86		Brown SILT (TOPSOIL)
								Firm brown gravelly CLAY.
					0.60	84.41		Medium dense silty clayey fine to coarse angular GRAVEL with medium cobble content. Highly weathered bedrock (Distinctly / residual soil).
					0.90	84.11		Extremely weak dark grey fractured muddy very fine grained GREYWACKE.
		0.97	B		0.97	84.04		End of Borehole at 0.970m

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

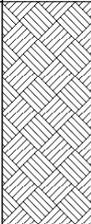
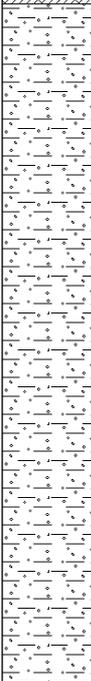
Remarks
No groundwater encountered.



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Trial Pit Log

Project Name: Kellystown Wind Farm		Client: EDF Renewables		Date: 29/04/2025	
Location:		Contractor:		Co-ords: E709341.00 N782586.20	
Project No. : 2200-23		Crew Name:		Equipment:	
Location Number TP-S6	Location Type TP	Level 89.18m AoD	Logged By	Scale 1:10	Page Number Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
					0.30	88.88		Brown SILT (TOPSOIL)
		1.00	B					Firm orange brown slightly gravelly CLAY. Very gravelly at 0.9 - 1.05m depth in the northern end of pit.
	▼				1.20	87.98		End of Borehole at 1.200m

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

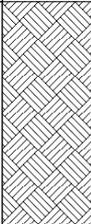
Remarks
Very minor seepage from the north wall of the trial pit at 1.2m depth.



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Trial Pit Log

Project Name: Kellystown Wind Farm		Client: EDF Renewables		Date: 29/04/2025	
Location:		Contractor:		Co-ords: E708869.20 N782760.40	
Project No. : 2200-23		Crew Name:		Equipment:	
Location Number TP-S7	Location Type TP	Level 95.93m AoD	Logged By	Scale 1:10	Page Number Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
					0.30	95.63		Brown SILT (TOPSOIL)
								Firm orange brown slightly gravelly CLAY
		0.90	B		0.90	95.03		End of Borehole at 0.900m

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks
No groundwater encountered.

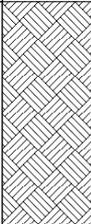
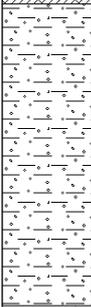


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Trial Pit Log

Project Name: Kellystown Wind Farm	Client: EDF Renewables	Date: 29/04/2025
Location:	Contractor:	Co-ords: E708886.90 N782733.40
Project No. : 2200-23	Crew Name:	Equipment:

Location Number TP-S7A	Location Type TP	Level 94.82m AoD	Logged By	Scale 1:10	Page Number Sheet 1 of 1
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Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
					0.30	94.52		Brown SILT (TOPSOIL)
					0.70	94.12		Firm orange brown slightly gravelly silty CLAY
								End of Borehole at 0.700m

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

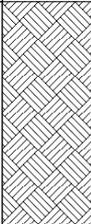
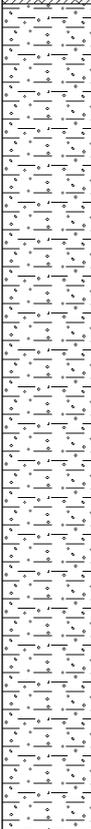
Remarks
No groundwater encountered.



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Trial Pit Log

Project Name: Kellystown Wind Farm		Client: EDF Renewables		Date: 29/04/2025	
Location:		Contractor:		Co-ords: E708890.90 N782734.60	
Project No. : 2200-23		Crew Name:		Equipment:	
Location Number TP-S7B	Location Type TP	Level 94.85m AoD	Logged By	Scale 1:10	Page Number Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
					0.30	94.55		Brown SILT (TOPSOIL)
								Firm brown slightly gravelly silty CLAY.
					1.40	93.45		End of Borehole at 1.400m

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks
No groundwater encountered.



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Infiltration Test

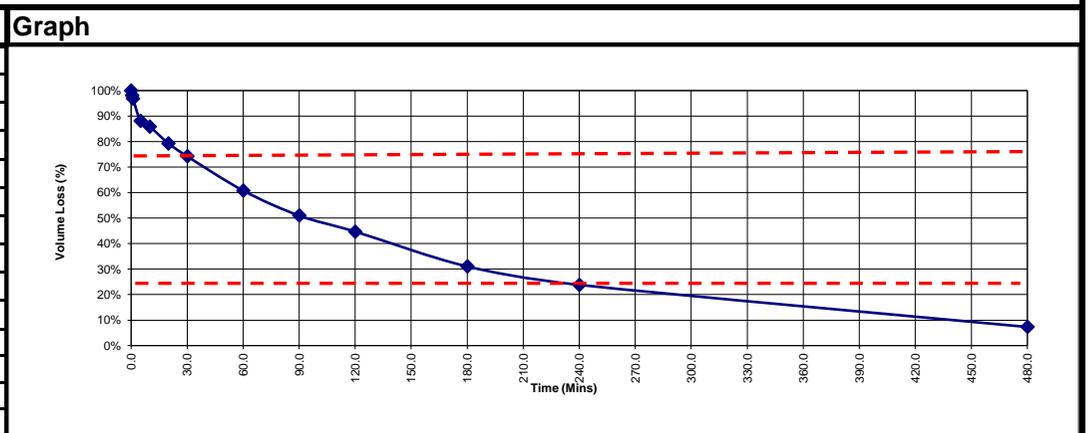
TP-S1

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Project:	Kellystown Wind Farm		
Job No:	2200/23		
Client:	EDF Renewables Ltd		
Survey Type:	Soakaway Test - (BRE Digest 365)		
GPS Co-ordinate:	E = 708151.7	N= 783111.3	
Date:	29/04/2025		
Soil Type:	Brown and orange brown CLAY		
Site Description:	Agricultural field, grass covered.		
Topography of Land:	Generally level ground		
Distance to nearest Watercourse:			
Height difference between Site and Watercourse:			
Description of Watercourse:	Drumshallon Lough Stream		
Catchment Area:			
Dimensions of Test (L/W/D) (m):	1.6	0.5	0.327
Base Dimensions of Test Hole (LxW) (m):	0.9	0.5	Volume (m ³)= 0.20

WATER DEPTH vs TIME

Depth To Water (m)	Time (mins)	(%)
0.000	0.0	100.00%
0.005	0.5	98.05%
0.008	1.0	96.89%
0.031	5.0	88.12%
0.037	10.0	85.88%
0.055	20.0	79.26%
0.069	30.0	74.24%
0.108	60.0	60.78%
0.138	90.0	50.97%
0.158	120.0	44.69%
0.204	180.0	31.04%
0.230	240.0	23.82%
0.295	480.0	7.31%



Additional Remarks:			
Refer to BRE Digest 365 for test procedure	$V_{p75-25} = 0.1021875 \text{ m}^3$	$T_{p75-25} = 180 \text{ mins}$	
Soakaway Test - Filled to ground level	$A_{p50} = 0.817875 \text{ m}^2$		
Average Percolation Rate =	1.1569E-05 m/s		
	MODERATE		Initials: JW

Infiltration Test

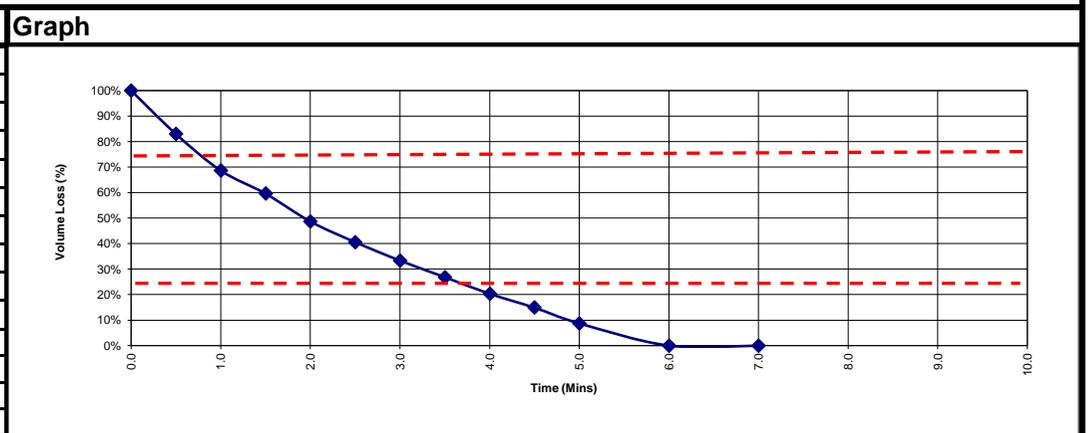
TP-S2

RECEIVED: 07/08/2025

Project:	Kellystown Wind Farm		
Job No:	2200/23		
Client:	EDF Renewables Ltd		
Survey Type:	Soakaway Test - (BRE Digest 365)		
GPS Co-ordinate:	E = 708019.6	N= 783272.8	
Date:	30/04/2025		
Soil Type:	Slightly sandy gravelly SILT		
Site Description:	Agricultural field, grass covered.		
Topography of Land:	Ground is level.		
Distance to nearest Watercourse:			
Height difference between Site and Watercourse:			
Description of Watercourse:	Drumshallon Lough Stream		
Catchment Area:			
Dimensions of Test (L/W/D) (m):	1.6	0.5	0.227
Base Dimensions of Test Hole (LxW) (m):	0.8	0.5	
			Volume (m ³)= 0.14

WATER DEPTH vs TIME

Depth To Water (m)	Time (mins)	(%)
0.000	0.0	100.00%
0.030	0.5	82.96%
0.057	1.0	68.62%
0.075	1.5	59.59%
0.098	2.0	48.65%
0.116	2.5	40.57%
0.133	3.0	33.32%
0.149	3.5	26.84%
0.166	4.0	20.32%
0.181	4.5	14.88%
0.199	5.0	8.73%
0.227	6.0	0.00%
0.227	7.0	0.00%



Additional Remarks:			
Refer to BRE Digest 365 for test procedure	V _{p75-25} =	0.0681 m ³	T _{p75-25} = 2.8 mins
Soakaway Test - Filled to ground level	A _{p50} =	0.6497 m ²	
Average Percolation Rate =	6.2391E-04 m/s		
	MODERATE TO HIGH		Initials: JW

Infiltration Test

TP-S3

RECEIVED: 07/08/2025

Project:	Kellystown Wind Farm		
Job No:	2200/23		
Client:	EDF Renewables Ltd		
Survey Type:	Soakaway Test - (BRE Digest 365)		
GPS Co-ordinate:	E = 709117.05	N= 783212	
Date:	30/04/2025		
Soil Type:	Brown and orange brown CLAY		
Site Description:	Agricultural field with barley crop.		
Topography of Land:	Ground is generally level.		
Distance to nearest Watercourse:			
Height difference between Site and Watercourse:			
Description of Watercourse:	Drumshallon Lough Stream		
Catchment Area:			
Dimensions of Test (L/W/D) (m):	1.6	0.5	0.321
Base Dimensions of Test Hole (LxW) (m):	1.2	0.5	
			Volume (m ³)= 0.22

WATER DEPTH vs TIME

Depth To Water (m)	Time (mins)	(%)	Graph
0.000	0.0	100.00%	
0.001	0.5	99.64%	
0.002	1.0	99.29%	
0.004	5.0	98.58%	
0.004	10.0	98.58%	
0.005	20.0	98.22%	
0.006	30.0	97.87%	
0.012	60.0	95.75%	
0.025	90.0	91.19%	
0.050	120.0	82.55%	
0.15	240.0	49.71%	
0.20	480.0	34.34%	
0.24	720.0	22.54%	

Additional Remarks:

Refer to BRE Digest 365 for test procedure	$V_{p75-25} =$	0.11235 m ³	$T_{p75-25} =$	545 mins
Soakaway Test - Filled to ground level	$A_{p50} =$	0.9852 m ²		
Average Percolation Rate =		3.4874E-06 m/s		

Test Abandoned due to low percolation rate.

MODERATE

Initials: JW

Infiltration Test

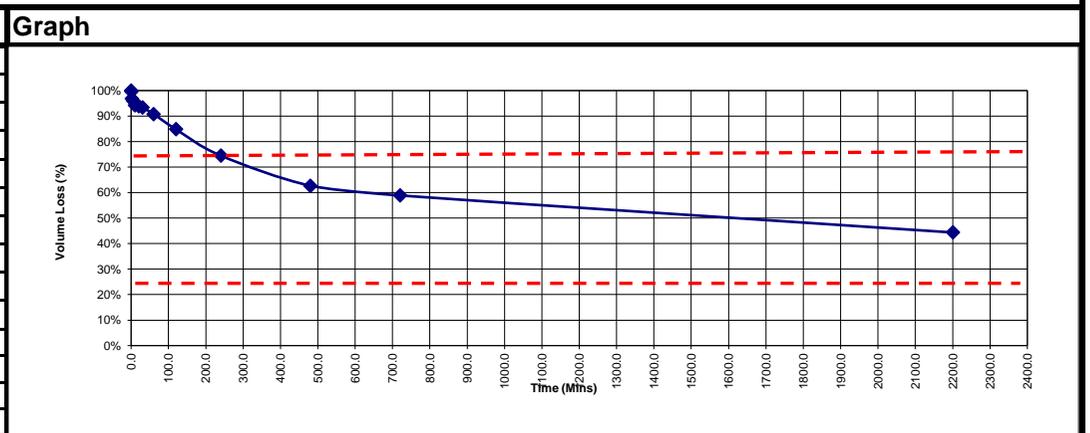
TP-S4

RECEIVED: 07/08/2025

Project:	Kellystown Wind Farm		
Job No:	2200/23		
Client:	EDF Renewables Ltd		
Survey Type:	Soakaway Test - (BRE Digest 365)		
GPS Co-ordinate:	E = 709163.8	N= 783225.2	
Date:	29/04/2025		
Soil Type:	Brown and orange brown CLAY		
Site Description:	Agricultural field with barley crop.		
Topography of Land:	Ground is generally level.		
Distance to nearest Watercourse:			
Height difference between Site and Watercourse:			
Description of Watercourse:	Drumshallon Lough Stream		
Catchment Area:			
Dimensions of Test (L/W/D) (m):	1.9	0.5	0.289
Base Dimensions of Test Hole (LxW) (m):	1.05	0.5	
			Volume (m ³)= 0.21

WATER DEPTH vs TIME

Depth To Water (m)	Time (mins)	(%)
0.000	0.0	100.00%
0.001	0.5	99.55%
0.007	1.0	96.90%
0.009	5.0	96.02%
0.013	10.0	94.26%
0.014	20.0	93.83%
0.015	30.0	93.39%
0.021	60.0	90.79%
0.04	120.0	84.82%
0.06	240.0	74.50%
0.09	480.0	62.68%
0.10	720.0	58.88%
0.14	2200.0	44.36%



Additional Remarks:			
Refer to BRE Digest 365 for test procedure	V _{p75-25} = 0.10656875 m ³	T _{p75-25} = N/A mins	
Soakaway Test - Filled to ground level	A _{p50} = 0.8826375 m ²		
Average Percolation Rate =	< 1 x 10 ⁻¹⁰ m/s		
Test Abandoned due to low percolation rate.	POOR	Initials: JW	

Infiltration Test

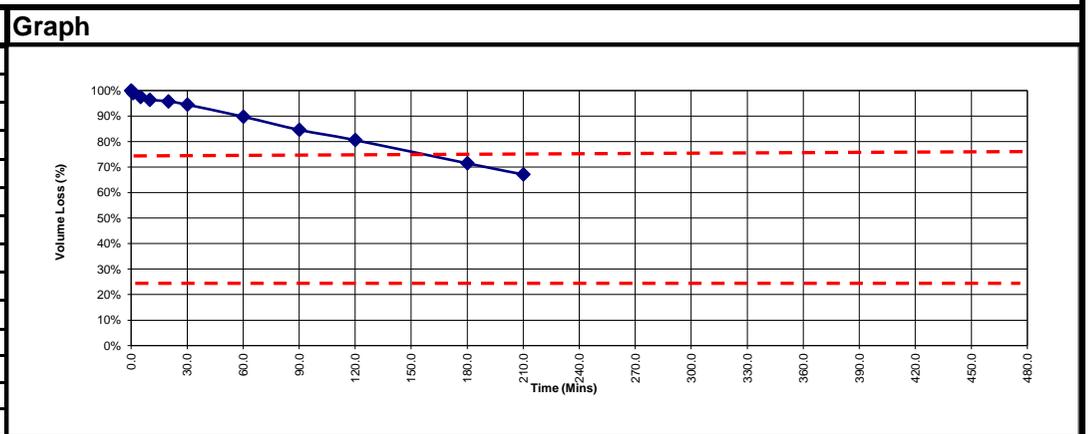
TP-S5

RECEIVED: 07/08/2025

Project:	Kellystown Wind Farm		
Job No:	2200/23		
Client:	EDF Renewables Ltd		
Survey Type:	Soakaway Test - (BRE Digest 365)		
GPS Co-ordinate:	E = 707989.4	N= 784924.9	
Date:	29/04/2025		
Soil Type:	Brown clayey GRAVEL.		
Site Description:	Grass field.		
Topography of Land:	Gently sloping to the north.		
Distance to nearest Watercourse:			
Height difference between Site and Watercourse:			
Description of Watercourse:	Drumshallon Lough Stream		
Catchment Area:			
Dimensions of Test (L/W/D) (m):	2.6	0.55	0.629
Base Dimensions of Test Hole (LxW) (m):	1.7	0.55	
			Volume (m ³)= 0.74

WATER DEPTH vs TIME

Depth To Water (m)	Time (mins)	(%)
0.000	0.0	100.00%
0.002	0.5	99.62%
0.005	1.0	99.04%
0.013	5.0	97.51%
0.019	10.0	96.37%
0.022	20.0	95.80%
0.029	30.0	94.47%
0.054	60.0	89.77%
0.082	90.0	84.59%
0.104	120.0	80.58%
0.155	180.0	71.47%
0.180	210.0	67.11%



Additional Remarks:			
Refer to BRE Digest 365 for test procedure	V _{p75-25} =	0.37189625 m ³	T _{p75-25} = mins
Soakaway Test - Filled to ground level	A _{p50} =	1.957125 m ²	
Average Percolation Rate =		m/s	
Test stopped due to slow percolation rate.			Initials: JW

Infiltration Test

TP-S5A

RECEIVED: 07/08/2025

Project:	Kellystown Wind Farm		
Job No:	2200/23		
Client:	EDF Renewables Ltd		
Survey Type:	Soakaway Test - (BRE Digest 365)		
GPS Co-ordinate:	E =	707986.7	N= 784927.8
Date:	01/05/2025		
Soil Type:	Distinctly weathered extremely weak fractured shaley MUDSTONE		
Site Description:	Grass field.		
Topography of Land:	Gently sloping to the north.		
Distance to nearest Watercourse:			
Height difference between Site and Watercourse:			
Description of Watercourse:	Drumshallon Lough Stream		
Catchment Area:			
Dimensions of Test (L/W/D) (m):	1.8	0.55	0.221
Base Dimensions of Test Hole (LxW) (m):	1.7	0.55	Volume (m ³)= 0.21

WATER DEPTH vs TIME

Depth To Water (m)	Time (mins)	(%)	Graph
0.000	0.0	100.00%	
0.002	0.5	99.07%	
0.002	1.0	99.07%	
0.002	5.0	99.07%	
0.015	10.0	93.03%	
0.042	20.0	80.56%	
0.062	30.0	71.37%	
0.123	60.0	43.64%	
0.188	90.0	14.57%	
0.211	105.0	4.40%	

Additional Remarks:			
Refer to BRE Digest 365 for test procedure	$V_{p75-25} =$	0.10635625 m ³	$T_{p75-25} =$ 51 mins
Soakaway Test - Filled to ground level	$A_{p50} =$	1.249925 m ²	
Average Percolation Rate =	2.7807E-05 m/s		
MODERATE			Initials: JW

Infiltration Test



RECEIVED: 07/08/2025

Project:	Kellystown Wind Farm		
Job No:	2200/23		
Client:	EDF Renewables Ltd		
Survey Type:	Soakaway Test - (BRE Digest 365)		
GPS Co-ordinate:	E = 709341	N= 782586.2	
Date:	28/4/2025		
Soil Type:	Firm orange brown slightly gravelly CLAY		
Site Description:	Grass field.		
Topography of Land:	The test location is relatively level. But it is the low point of the surrounding areahhh		
Distance to nearest Watercourse:			
Height difference between Site and Watercourse:			
Description of Watercourse:	Pipperstown Stream		
Catchment Area:			
Dimensions of Test (L/W/D) (m):	1.5	0.5	0.2
	Volume (m ³)= 0.13		
Base Dimensions of Test Hole (LxW) (m):	1	0.5	
WATER DEPTH vs TIME			
Depth To Water (m)	Time (mins)	(%)	Graph
0.00	0.0	100.00%	
0.00	0.5	100.00%	
0.02	1.0	91.11%	
0.05	5.0	69.60%	
0.07	10.0	62.04%	
0.08	20.0	57.29%	
0.09	30.0	52.10%	
0.12	60.0	37.13%	
0.13	120.0	31.39%	
0.13	240.0	28.58%	
0.14	300.0	26.72%	
Additional Remarks: Test stopped after 5 hours due to end of shift and unable to leave excavation open overnight.			
Refer to BRE Digest 365 for test procedure	$V_{p75-25} =$	0.0625 m ³	$T_{p75-25} =$ 325 mins
Soakaway Test - Filled to ground level	$A_{p50} =$	0.725 m ²	
Average Percolation Rate =	4.4209E-06 m/s		
MODERATE			Initials: JW

Infiltration Test

TP-S7

RECEIVED: 07/08/2025

Project:	Kellystown Wind Farm		
Job No:	2200/23		
Client:	EDF Renewables Ltd		
Survey Type:	Soakaway Test - (BRE Digest 365)		
GPS Co-ordinate:	E = 708869.2	N= 782760.4	
Date:	29/04/2025		
Soil Type:	Firm brown CLAY		
Site Description:	Agricultural field. Grass crop.		
Topography of Land:	Gently sloping to the south.		
Distance to nearest Watercourse:			
Height difference between Site and Watercourse:			
Description of Watercourse:	Pipperstown Stream		
Catchment Area:			
Dimensions of Test (L/W/D) (m):	1.9	0.5	0.3
Base Dimensions of Test Hole (LxW) (m):	1.2	0.5	
			Volume (m ³)= 0.19

WATER DEPTH vs TIME

Depth To Water (m)	Time (mins)	(%)	Graph
0.000	0.0	100.00%	
0.001	0.5	99.49%	
0.001	1.0	99.49%	
0.016	5.0	91.97%	
0.018	10.0	90.98%	
0.018	20.0	90.98%	
0.018	30.0	90.98%	
0.018	60.0	90.98%	

Additional Remarks:			
Refer to BRE Digest 365 for test procedure	$V_{p75-25} =$	0.09375 m ³	$T_{p75-25} =$ mins
Soakaway Test - Filled to ground level	$A_{p50} =$	0.9825 m ²	
Average Percolation Rate =		m/s	
Test abandoned due to very low percolation rate.			Initials: JW

Infiltration Test

TP-S7A

RECEIVED: 07/08/2025

Project:	Kellystown Wind Farm		
Job No:	2200/23		
Client:	EDF Renewables Ltd		
Survey Type:	Soakaway Test - (BRE Digest 365)		
GPS Co-ordinate:	E =	708886	N= 782733
Date:	29/04/2025		
Soil Type:	Firm brown CLAY		
Site Description:	Agricultural field. Grass crop.		
Topography of Land:	Gently sloping of the south.		
Distance to nearest Watercourse:			
Height difference between Site and Watercourse:			
Description of Watercourse:	Pipperstown Stream		
Catchment Area:			
Dimensions of Test (L/W/D) (m):	1.95	0.5	0.225
Base Dimensions of Test Hole (LxW) (m):	1.3	0.5	
			Volume (m ³)= 0.14

WATER DEPTH vs TIME

Depth To Water (m)	Time (mins)	(%)	Graph
0.000	0.0	100.00%	
0.005	0.5	96.55%	
0.005	1.0	96.55%	
0.012	5.0	91.75%	
0.014	10.0	90.39%	
0.014	20.0	90.39%	
0.014	30.0	90.39%	

Additional Remarks:			
Refer to BRE Digest 365 for test procedure	$V_{p75-25} =$	0.0703125 m ³	$T_{p75-25} =$ mins
Soakaway Test - Filled to ground level	$A_{p50} =$	0.9453125 m ²	
Average Percolation Rate =		m/s	
Test abandoned due to very low percolation rate.			Initials: JW

Infiltration Test

TP-S7B

RECEIVED: 07/08/2025

Project:	Kellystown Wind Farm		
Job No:	2200/23		
Client:	EDF Renewables Ltd		
Survey Type:	Soakaway Test - (BRE Digest 365)		
GPS Co-ordinate:	E = 708890.9	N= 782734.6	
Date:	29/04/2025		
Soil Type:	Firm brown CLAY		
Site Description:	Agricultural field. Grass crop.		
Topography of Land:	Gently sloping ot the south.		
Distance to nearest Watercourse:			
Height difference between Site and Watercourse:			
Description of Watercourse:	Pipperstown Stream		
Catchment Area:			
Dimensions of Test (L/W/D) (m):	1.4	0.5	0.364
Base Dimensions of Test Hole (LxW) (m):	1.1	0.5	
			Volume (m ³)= 0.23

WATER DEPTH vs TIME

Depth To Water (m)	Time (mins)	(%)	Graph
0.000	0.0	100.00%	
0.005	0.5	98.46%	
0.007	1.0	97.85%	
0.012	5.0	96.32%	
0.014	10.0	95.71%	
0.015	20.0	95.40%	
0.017	30.0	94.80%	
0.018	60.0	94.49%	

Additional Remarks:			
Refer to BRE Digest 365 for test procedure	V _{p75-25} =	0.11375 m ³	T _{p75-25} = mins
Soakaway Test - Filled to ground level	A _{p50} =	0.9595 m ²	
Average Percolation Rate =	< 1 x 10 ⁻¹⁰	m/s	
Test abandoned due to Impermeability	POOR		Initials: JW

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APPENDIX C

SOAKAWAY TEST SPECIFICATION

BRE Digest 365 – “Soakaway Design”

8 x A4

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Soakaways have been the traditional way to dispose of stormwater from buildings and paved areas remote from a public sewer or watercourse. In recent years, soakaways have been used within urban, fully-sewered areas to limit the impact on discharge of new upstream building works and to avoid costs of sewer upgrading outside a development. Soakaways are seen increasingly as a more widely applicable option alongside other means of stormwater control and disposal.

Soakaways must store the immediate stormwater run-off and allow for its efficient

infiltration into the adjacent soil. They must discharge their stored water sufficiently quickly to provide the necessary capacity to receive run-off from a subsequent storm. The time taken for discharge depends upon the soakaway shape and size, and the surrounding soil's infiltration characteristics. They can be constructed in many different forms and from a range of materials.

This Digest describes design and construction procedures, explains how to calculate rainfall design values and soil infiltration rates, and gives design examples.

Shape and size

Soakaways for areas less than 100 m² have traditionally been built as square or circular pits, either filled with rubble or lined with dry-jointed brickwork or pre-cast perforated concrete ring units surrounded by suitable granular backfill. BS 8301 suggests that soakaways may take the form of trenches that follow convenient contours: compared with square or circular shapes, they have larger internal surface areas for infiltration of stormwater for a given stored volume. The designer must consider the merits of the more compact square or circular forms against the better rate of discharge from the trench in the particular conditions of soil type, available space, site layout and topography.

For drained areas above 100 m², soakaways can be pre-cast ring or of trench type and not substantially deeper than soakaways that serve small areas: 3 to 4 m is adequate if ground conditions allow. Although limiting the depth does mean the length must be increased, trench soakaways are cheaper to dig with readily available excavating equipment.

Soil infiltration characteristics

The method of determination must give representative results for the proposed site of the soakaway. This is achieved by:

- 1** Excavating a trial pit of sufficient size to represent a section of the design soakaway.
- 2** Filling the pit several times in quick succession whilst monitoring the rate of seepage, to represent soil moisture conditions typical of the site when the soakaway becomes operative.
- 3** Examining site data to ensure that variations in soil conditions, areas of filled land, preferential underground seepage routes, variations in the level of groundwater, and any geotechnical and geological factors likely to affect the long-term percolation and stability of the area surrounding the soakaway have been assessed. Groundwater should not rise to the level of the base of the soakaway during annual variations in the water table. Local building control and/or planning authorities should advise where fluctuations in groundwater level may cause a problem in the long-term for any proposed depth of excavation.



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Design procedure

The design method for sizing a soakaway is based upon the equation of volumes:

$$I - O = S$$

where:

I = the inflow from the impermeable area drained to the soakaway;

O = the outflow infiltrating into the soil during rainfall;

S = the required storage in the soakaway to balance temporarily inflow and outflow.

Inflow to the soakaway

$$I = A \times R$$

where:

A = the impermeable area drained to the soakaway;

R = the total rainfall in a design storm (a 10-year return period should be used); calculation of R is shown in the box below.

Outflow from the soakaway

$$O = a_{50} \times f \times D$$

where:

a_{50} = the internal surface area of the soakaway to 50% effective depth: this excludes the base area which is assumed to clog with fine particles and become ineffective in the long-term;

f = the soil infiltration rate determined in a trial pit at the site of the soakaway;

D = the storm duration.

Required storage volume in the soakaway, S

Storage must equal or be greater than inflow minus outflow, defined above, and is the required effective volume available between the base of the soakaway and the invert of the drain discharging to the soakaway.

There are four steps in the design procedure:

- 1 carry out a site investigation to determine the soil infiltration rate;
- 2 decide on a construction type (eg filled pit in square, circular or trench form, or concrete ring units with granular surround);
- 3 calculate required storage volume, S , from inflow minus outflow for a range of durations of 10-year design storms to determine the maximum storage predicted for the type of soakaway;
- 4 review the design to ensure its overall suitability considering space requirements, site layout and time for emptying.

This design method for sizing soakaways contains assumptions which generally combine to increase the factor of safety against surface flooding of the design:

- the percentage run-off is taken as 100% from the drained area, ie no reduction is made to the design run-off volume discharged to the soakaway for losses due to surface wetting or the filling of puddles during the storm;
- no allowance is made for the time taken for run-off to discharge to the soakaway: the required storage volume is calculated on the basis of instantaneous discharge to the soakaway;
- the outflow from the soakaway is under-estimated; higher infiltration rates occur at greater depths of storage in practice than are adopted in design, and because the outflow is calculated on the basis of the rainfall duration rather than the run-off duration. The latter may be considerably longer, depending on the length of drains.

Calculating design rainfall

Values of design rainfall, R , can be determined using Figure 1 and Tables 1 and 2 for different storm durations with a 10-year return period. The notation

$MX-D$ min

is used to identify the storm, where:

X = the return period in years;

D = the storm duration in minutes.

The 10-year return period rainfall of 15 minute duration, known as M10-15 min, or the M10-30 min rainfall, is calculated as follows:

From the map in Figure 1, determine the rainfall ratio, r , for the location of the soakaway (interpolating between contours). Use this in Table 1 to give the factor $Z1$ for the calculation of the 5-year return period rainfall total, M5- D min, for different storm durations, D .

The basis of the calculation is the M5-60 min rainfall: this can be taken to be 20 mm for all parts of the United Kingdom.

$$\begin{aligned} \text{M5-}D \text{ min rainfall} &= \text{M5-60 min rainfall} \times Z1 \\ &= 20 \text{ mm} \times Z1 \end{aligned}$$

$$\text{M10-}D \text{ min} = \text{M5-}D \text{ min} \times Z2$$

where $Z2$ is found from Table 2.

For example,

if, for the soakaway location, r on Figure 1 = 0.42, the M5-15 min can be found as follows:

$$\text{M5-15 min rainfall} = 20 \text{ mm} \times Z1 \text{ (for 15 min duration)}$$

Read $Z1$ from Table 1 in the column for the required rainfall duration, D , (eg 15 minutes), and interpolate for the appropriate rainfall ratio, r , at the site: (eg $D = 15$ min; $r = 0.42$; $Z1 = 0.64$)

$$= 20 \text{ mm} \times 0.64$$

$$\text{M5-15 min rainfall} = 12.8 \text{ mm}$$

$$\begin{aligned} \text{M5-30 min rainfall} &= \text{M5-60 min rainfall} \times Z1 \text{ (for 30 min duration)} \\ &= 20 \text{ mm} \times 0.81 \\ &= 16.2 \text{ mm} \end{aligned}$$

The required 10-year return period rainfalls used in the soakaway design are calculated by interpolating the growth factors $Z2$ from Table 2.

For example,

$$\begin{aligned} \text{M10-15 min rainfall} &= \text{M5-15 min rainfall} \times Z2 \\ &= 12.8 \text{ mm} \times 1.23 \text{ (for England and Wales)} \\ &= 15.7 \text{ mm} \end{aligned}$$

$$\begin{aligned} \text{M10-30 min rainfall} &= 16.2 \text{ mm} \times 1.24 \\ &= 20.1 \text{ mm} \end{aligned}$$

This procedure to determine the 10-year rainfalls must be used because the basic data are available only for 5-year returns.

Table 1 Values of Factor Z1 for rainfall duration D and ratio r

r	Rainfall duration D									
	Minutes				Hours					
	5	10	15	30	1	2	4	6	10	24
0.12	0.22	0.34	0.45	0.67	1.00	1.48	2.17	2.75	3.70	6.00
0.15	0.25	0.38	0.48	0.69	1.00	1.42	2.02	2.46	3.23	4.90
0.18	0.27	0.41	0.51	0.71	1.00	1.36	1.86	2.25	2.86	4.30
0.21	0.29	0.43	0.54	0.73	1.00	1.33	1.77	2.12	2.62	3.60
0.24	0.31	0.46	0.56	0.75	1.00	1.30	1.71	2.00	2.40	3.35
0.27	0.33	0.48	0.58	0.76	1.00	1.27	1.64	1.88	2.24	3.10
0.30	0.34	0.49	0.59	0.77	1.00	1.25	1.57	1.78	2.12	2.84
0.33	0.35	0.50	0.61	0.78	1.00	1.23	1.53	1.73	2.04	2.60
0.36	0.36	0.51	0.62	0.79	1.00	1.22	1.48	1.67	1.90	2.42
0.39	0.37	0.52	0.63	0.80	1.00	1.21	1.46	1.62	1.82	2.28
0.42	0.38	0.53	0.64	0.81	1.00	1.20	1.42	1.57	1.74	2.16
0.45	0.39	0.54	0.65	0.82	1.00	1.19	1.38	1.51	1.68	2.03

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Soil infiltration rate

Excavate a soakage trial pit to the same depth as anticipated in the full-size soakaway; for run-off from 100 m² this will be 1 to 1.5 m below the invert level of the drain discharging to the soakaway. Overall depths of excavation will be typically 1.5 to 2.5 m for permeable areas up to 100 m² draining to the soakaway.

The trial pit should be 0.3 to 1 m wide and 1 to 3 m long. It should have vertical sides trimmed square and, if necessary for stability, should be filled with granular material. When granular fill is used, a full-height, perforated, vertical observation tube should be positioned in the pit so that water levels can be monitored with a dip tape. It should be possible to construct a suitably dimensioned pit with a backhoe loader or mini-excavator. Narrow, short pits use less water for the soakage tests but may be more difficult to trim and clean prior to testing. Measure the pit carefully before trials. **For safety reasons do not enter the pit.**

A lot of water will be used to determine the soil infiltration rate so a water bowser may be needed. The inflow should be rapid so that the pit can be filled to its maximum effective depth in a short time, ie to the design invert level of the drain to the soakaway. Take care that the inflow does not cause the walls of the pit to collapse. Fill the pit and allow it to drain three times to near

empty; each time record the water level and time from filling, at intervals sufficiently close to clearly define water level versus time (Figure 2). The three fillings should be on the same or consecutive days.

Calculate the soil infiltration rate from the time taken for the water level to fall from 75% to 25% effective storage depth in the pit, using the lowest f value of the three test results for design:

$$\text{Soil infiltration rate, } f = \frac{V_{p75-25}}{a_{p50} \times t_{p75-25}}$$

where:

V_{p75-25} = the effective storage volume of water in the trial pit between 75% and 25% effective depth;

a_{p50} = the internal surface area of the trial pit up to 50% effective depth and including the base area;

t_{p75-25} = the time for the water level to fall from 75% to 25% effective depth.

If the test pit is deeper than about 3 m, it may be difficult to supply sufficient water for a full-depth soakage test. Tests may be conducted at less than full depth but determinations of the soil infiltration rate may be lower than those from the full-depth test. This is because relationships between depth of water in the soakage pit, the effective area of outflow and the infiltration rate can vary with depth, even when soil conditions themselves do not vary. The variation in infiltration rate with the depth at which the determination is made may be as much as a factor of two. From the results of a soakage trial in Figure 2, the calculated infiltration rate based upon a fall of water level from:

- 75% to 50% effective depth is 5.1×10^{-5} m/s;
- 50% to 25% effective depth is 2.9×10^{-5} m/s.

The design method adopts the result determined from 75% to 25% effective depth of 3.3×10^{-5} m/s.

If it is impossible to carry out a full-depth soakage test, soil infiltration rate calculation should be based on the time for fall of water level from 75% to 25% of the actual

Table 2 Growth Factor Z2 for M10 rainfalls from M5 rainfalls

M5 rainfall mm	M10 Growth Factor Z2	
	England and Wales	Scotland and N Ireland
5	1.19	1.17
10	1.22	1.19
15	1.24	1.20
20	1.24	1.19
25	1.24	1.18
30	1.22	1.18
40	1.19	1.17
50	1.17	1.16
75	1.14	1.14
100	1.13	1.13

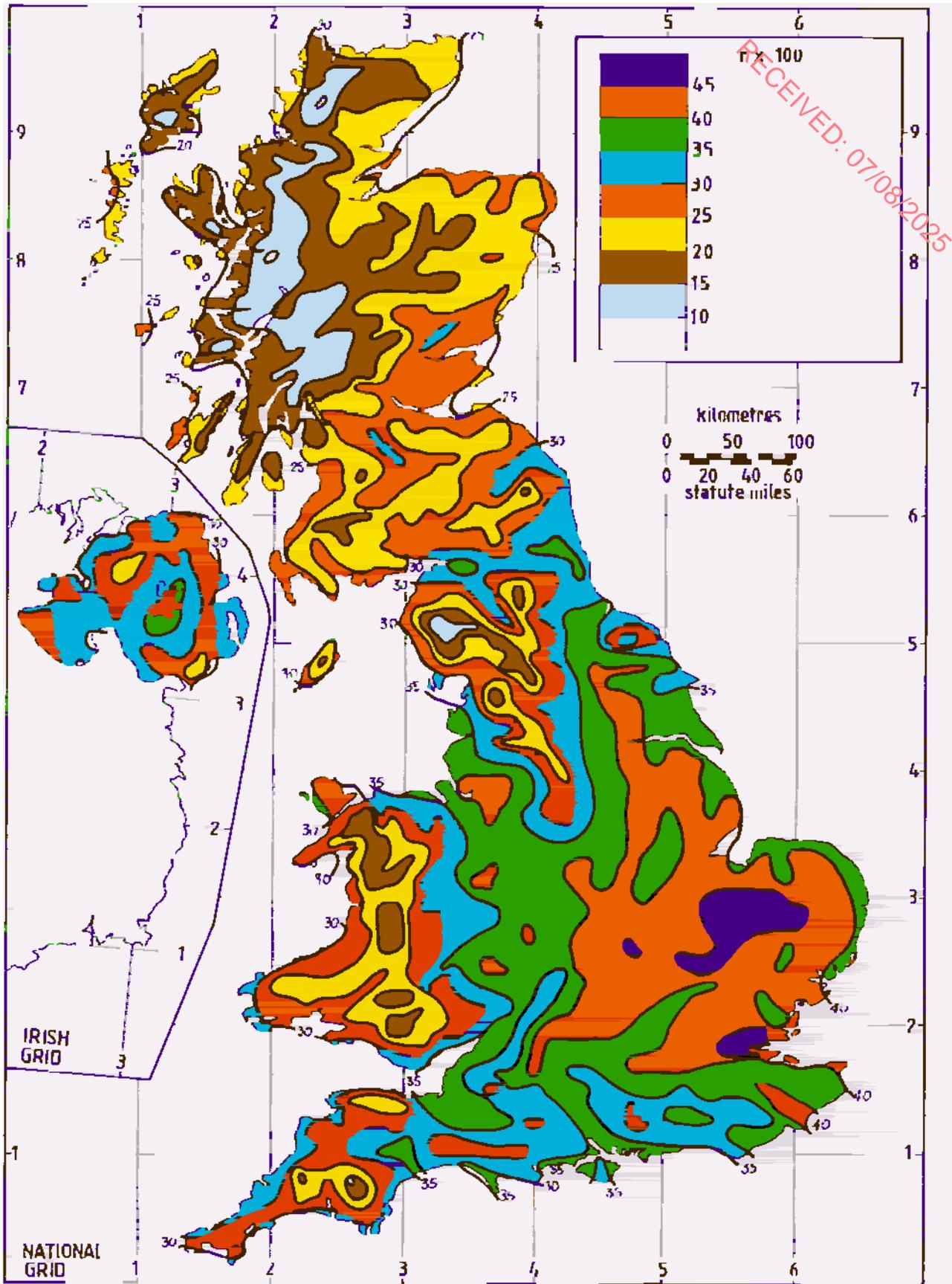


Figure 1 Ratio of 60-minute to 2-day rainfalls of 5-year return period – from *Design and analysis of urban storm damage*

maximum water depth achieved in the test. The effective area of loss from the soakage pit is then calculated as the internal surface area of the pit to 50% maximum depth achieved plus the base area of the pit.

In general, soakage trials should be undertaken where

the drain will discharge to the soakaway. The use of full-depth and of repeat determinations at locations along the line of trench soakaways is very important when soil conditions vary; if the soil is fissured, infiltration rates can vary enormously. In these situations, a preliminary design

Calculating soil infiltration rate

Figure 2 shows typical field observations from a soakage trial pit. It was known that the invert of the discharge drain was to be 1.0 m below ground surface. An effective storage depth of 1.5 m was adopted. When trimmed and clean, the trial pit was 2.51 m deep, 2.40 m long and 0.60 m wide

$$\text{Volume outflowing between 75\% and 25\% effective depth: } V_{p75-25} \\ = 2.40 \times 0.60 \times (2.13 - 1.38) = 1.09 \text{ m}^3$$

The mean surface area through which the outflow occurs, taken to be the pit sides to 50% effective depth and including the base of the pit: a_{p50}

$$= (2.40 \times 0.755 \times 2) + (0.6 \times 0.755 \times 2) + (2.40 \times 0.60) \\ = 5.97 \text{ m}^2$$

From Figure 2, the time for the outflow between 75% and 25% effective depth: t_{p75-25}

$$= 102 - 11 = 91 \text{ minutes}$$

$$\text{Soil infiltration rate, } f = \frac{1.09}{5.97 \times 91 \times 60} = 3.3 \times 10^{-5}$$

length for the proposed soakaway should be calculated from the first soakage trial pit result and, if the design length exceeds 10 m, a second trial should be carried out at the design length distance along the line of the soakaway. In all ground conditions, a second trial pit should be dug if the trench soakaway (designed on the basis of one trial pit) is longer than 25 m; further trial pits are needed at intervals of 25 m along the line of a long soakaway. If more than one trial pit is used, the mean value of the soil percolation rates determined from the trial pits is adopted for the final design

Time of emptying of soakaway

The soakaway should discharge from full to half-volume within 24 hours in readiness for subsequent storm inflow.

Construction details

Maintenance of soakaways has always presented problems, usually in finding them! This is certainly the case with rubble-filled ones. All soakaways should be provided with some form of inspection access, so that the point of discharge of the drain to the soakaway can be seen. This access will identify the location and will allow material to be cleared from the the soakaway.

Little monitoring of soakaway performance is done, but this could be most informative about changes in soil infiltration rate and in warning of soakaway blockage in the long-term. The inspection access should provide a clear view to the base of the soakaway, even when the soakaway is of the filled type (Figure 3). For small, filled soakaways, a 225 mm perforated pipe provides a suitable inspection well. Lined soakaways have the advantage of access for inspection and cleaning and this should be a feature of all soakaways. Trench-type soakaways should have at least two inspection access points, one at each end of a straight trench, with a horizontal perforated or

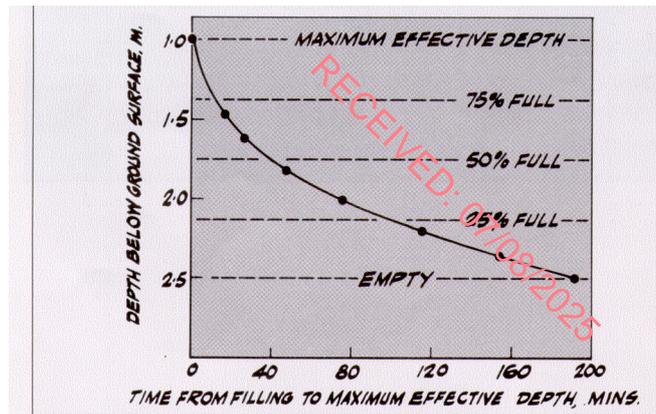


Figure 2 Field observations from a soakage trial pit
2.51 m deep; 2.4 m long; 0.6 m wide – no granular fill

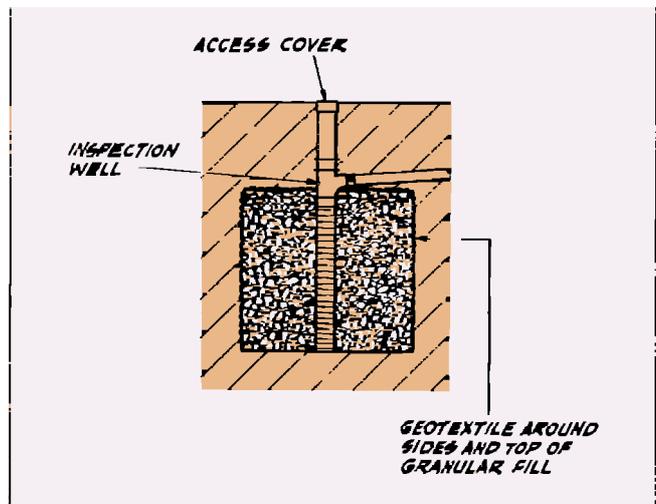


Figure 3 Small, filled soakaway with perforated inspection well extending to base of soakaway providing access to discharge drain outlet

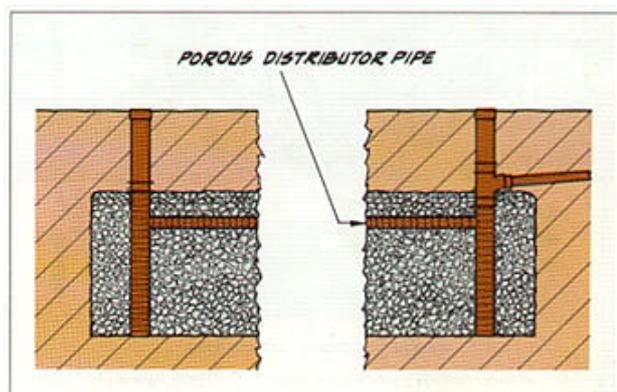


Figure 4 Trench type soakaway with horizontal distributor pipe

porous distributor pipe linking the ends along the top of the granular fill (Figure 4). It may be convenient with a trench soakaway to have several drain discharge points along the length of the trench, each connected to the soakaway via an inspection access chamber.

In trench soakaways, the movement of suspended and floating material into the distributor pipe can be minimised by using wet wells with a T-piece inlet fitted to the distributor pipe (Figure 5). Two or more T-piece inlets to distributor pipes in two or more trench

DESIGN EXAMPLES

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Design a soakaway to receive stormwater from 95 m² impermeable surface for a site near Southampton

Find the rainfall ratio from Figure 1 ($r = 0.35$) and calculate the storm rainfalls for a range of storm durations.

Storm duration D min	M5- D min = 20 mm x Z1	Z2	M10- D min = R mm
10	10.2	1.22	12.4
15	12.4	1.23	15.3
30	15.8	1.24	19.6
60	20.0	1.24	24.8
120	24.4	1.24	30.3
240	30.0	1.22	36.6
360	33.8	1.21	40.9
600	39.0	1.19	46.6

Assuming the results from a soakage trial pit (Figure 2) were obtained at the site, they can be used to design a soakaway which will be filled with granular material having 30% free volume. The percentage void space of any granular fill material must be pre-determined for use in the design method.

Take the soakaway dimensions as:

2.4 m long x 2.5 m deep x 1.5 m effective storage depth, so that the soakage trial pit can form part of the full-scale soakaway.

Calculate the design width of the soakaway:

Volume equation $I - O = S$

Inflow to soakaway I :

$$I = A \times R$$

= impermeable surface area x M10-D min rainfall

eg for 10 min storm duration, M10-10 min = 12.4 mm = 0.0124 m

$$I = 95 \times 0.0124 \\ = 1.178 \text{ m}^3$$

Outflow from soakaway O :

$$O = a_{s50} \times f \times D$$

= internal surface area of soakaway pit to 50% storage depth (excluding base area) x soil percolation rate x storm duration

For rectangular pit 2.4 m long x 1.5 m effective depth x W m wide

$$a_{s50} = 2 \times (2.4 + W) \times (1.5 \div 2) \\ = 3.6 + 1.5 W \text{ m}^2$$

$f = 3.3 \times 10^{-5}$ m/s from soakage trial

$$O = (3.6 + 1.5 W) \times (3.3 \times 10^{-5}) \times (D \times 60) \text{ m}^3$$

Soakaway storage volume S

$$= \text{effective volume of soakaway with 30\% free volume} \\ = 2.4 \times 1.5 \times W \times 0.3 \\ = 1.08 W \text{ m}^3$$

For satisfactory storage of the M10-10 min run-off.

$$I - O = S$$

$$1.178 - (3.6 + 1.5 W) \times (3.3 \times 10^{-5}) \times (10 \times 60) = 1.08 W$$

Required soakaway width $W = 1.00$ m

Repeat the calculation for a range of M10- D min storms and determine the maximum width. Results are summarised below.

Storm duration D - min	Required soakaway width W - m
10	1.00
15	1.20
30	1.41
60	1.53
120	1.41
240	0.99

A soakaway 2.4 m long x 1.5 m effective depth x 1.53 m wide would be suitable with the critical storm duration around one hour for 10-year events. The design might be suitable for the site layout but, if not, alternative shapes could be investigated. For example, if a narrow soakaway was necessary similar to the soakage trial pit (0.6 m wide x 1.5 m effective depth), calculations show that it must be 5.1 m long, with the critical storm duration around 30 mins.

Check on time of emptying half storage volume, t_{s50}

$$t_{s50} = \frac{S \times 0.5}{a_{s50} \times f} = \frac{(1.08 \times 1.53) \times 0.5}{(3.6 + [1.5 \times 1.53]) \times (3.3 \times 10^{-5})} \text{ seconds}$$

$$t_{s50} = 1.2 \text{ hours}$$

This design is clearly satisfactory but with soil infiltration rates of about 10^{-7} it may take days for the soakaway to half empty so the performance would be unsuitable.

Design an alternative soakaway for the site at Southampton using perforated concrete ring units

The rainfall results in Table 3 can be used again; the soil infiltration rate is 3.3×10^{-5} m/s and the effective depth of storage is 1.5 m. Use an initial design of 900 mm internal diameter concrete ring units, placed in a square pit of side length L , with granular backfill with 30% free volume between the rings and the sides of the pit.

Volume equation $I - O = S$

$$\begin{aligned} I &= A \times R \\ &= 95 \times 0.0124 \\ &= 1.178 \text{ m}^3 \text{ for M10-10 min storm} \\ O &= a_{s50} \times f \times D \end{aligned}$$

For a square soakaway with 1.5 m effective storage depth and excluding base area:

$$\begin{aligned} a_{s50} &= 4 \times L \times 1.5 \times 0.5 \\ &= 3L \text{ m}^2 \\ O &= 3L \times (3.3 \times 10^{-5}) \times (D \times 60) \text{ m}^3 \\ &= 0.0594L \text{ m}^3 \text{ for M10-10 min storm} \end{aligned}$$

Soakaway storage volume S

$$\begin{aligned} &= \text{free volume in granular fill} + \text{volume within concrete ring units} \\ \text{Volume within 900 mm ring units} &= 3.142 \times 0.45^2 \times 1.5 \\ &= 0.95 \text{ m}^3 \end{aligned}$$

Free volume in granular fill surrounding ring units in a square pit

$$\begin{aligned} &= (1.5L^2 - [3.142 \times 0.50^2 \times 1.5]) \times 0.3 \\ &= 0.45L^2 - 0.353 \text{ m}^3 \end{aligned}$$

(0.50 m = internal radius of the concrete ring plus 50 mm wall thickness)
Total volume $S = 0.95 + (0.45L^2 - 0.353)$
 $= 0.597 + 0.45L^2 \text{ m}^3$

For satisfactory storage of the M10-10 min run-off

$$I - O = S$$

$$\begin{aligned} 1.178 - 0.0594L &= 0.597 + 0.45L^2 \\ 0.45L^2 + 0.0594L - 0.581 &= 0 \end{aligned}$$

$$\text{Side length } L = \frac{-0.0594 + (0.0594^2 + 4 \times 0.45 \times 0.581)^{0.5}}{2 \times 0.45}$$

$$L = 1.07 \text{ m}$$

Repeat for a range of M10- D min storms and determine the maximum size of excavation. Results are summarised below.

Storm duration D – min	Required soakaway pit L – m
10	1.07
15	1.28
30	1.49
60	1.62
120	1.59
240	1.40

Choose a soakaway 1.62 m square subject to a check on time of emptying half the storage, t_{s50}

$$t_{s50} = \frac{S \times 0.5}{a_{s50} \times f} = \frac{(0.597 + 0.45(1.62)^2) \times 0.5}{(3 \times 1.62) \times (3.3 \times 10^{-5})} \text{ seconds} = 1.5 \text{ hours}$$

If the initial design using 900 mm concrete units and the calculation of the pit side length is unsatisfactory, select another standard size of unit and repeat the calculation.

Design a trench soakaway to receive stormwater run-off from 400 m² impermeable surface

Choose a trench 0.6 m wide, 1.5 m effective depth, with granular fill having 30% free volume.

Calculate the soakaway trench length, L . The rainfall ratio r is 0.35 and soil infiltration rate f is 3.3×10^{-5} m/s.

Volume equation $I - O = S$

$$\begin{aligned} I &= A \times R \\ &= 400 \times 0.0124 \\ &= 4.96 \text{ m}^3 \text{ for M10-10 min storm} \\ O &= a_{s50} \times f \times D \\ a_{s50} &= 2 \times (0.6 + L) \times (1.5 \div 2) \\ O &= (0.9 + 1.5L) \times (3.3 \times 10^{-5}) \times (D \times 60) \text{ m}^3 \\ &= 0.01782 + 0.0297L \text{ m}^3 \text{ for M10-10 min storm} \end{aligned}$$

Soakaway storage volume, S , = effective volume in trench with 30% free volume.

$$S = L \times 0.6 \times 1.5 \times 0.3 = 0.27L \text{ m}^3$$

For satisfactory storage of the M10-10 min run-off:

$$I - O = S$$

$$4.96 - 0.01782 - 0.0297L = 0.27L$$

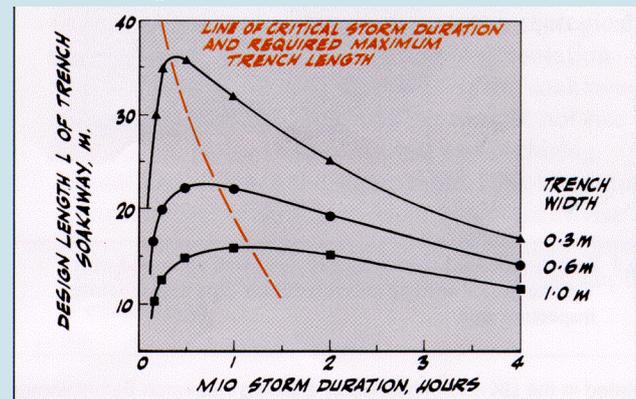
$$L = 16.5 \text{ m}$$

Repeat the calculation for a range of M10- D min storms and determine the maximum length. The results are summarised below.

Storm duration D – min	Required soakaway length L – m
10	16.5
15	19.7
30	21.9
60	21.9
120	19.2
240	14.0

A soakaway 22 m long, 1.5 m effective depth and 0.6 m wide is suitable; time for half emptying is 45 minutes. Such a design might be compatible with site layout and topography but an alternative trench cross-section could be investigated. Maintain the 1.5 m effective depth but use trench widths of 0.3 m and 1.0 m. The various design lengths of trench for the three widths are shown below for a range of 10-year return period storms. As the design width increases, the required length decreases and the critical storm duration increases. So if a design fails to meet the 24-hour time for half empty criterion, reducing the width and thereby increasing the length of a trench soakaway might achieve a satisfactory design. Similarly, if a design based upon a perforated pre-cast concrete ring unit soakaway fails the 24-hour criterion, a trench-type soakaway may be satisfactory.

With narrower, longer soakaways the volume of the soakaway trench is reduced relative to the wider trench designs: the required storage is reduced because of the enhanced outflow performance. In the figure below the volume of the trench designed 0.3 m wide is only 70% of a 1.0 m wide trench so there are savings in the cost of excavation and granular fill material.



Required design length of trench soakaway plotted against design storm duration for 10-year return storm periods

soakaways may be appropriate for large wet well designs. The advantages of sedimentation of fine material in the pre-cast chamber, for ease of maintenance and extended operating life, are combined with the more efficient trench discharge characteristics.

Perforated, pre-cast concrete ring unit soakaways should be installed within a square pit, with sides about twice the selected ring unit diameter. The need to oversize the soakaway pit for purposes of constructing the ring unit chamber may be used to advantage by incorporating the total excavation volume below the discharge drain invert in the design storage volume.

Granular material must be separated from the surrounding soil by a suitable geotextile to prevent migration of fines into the soakaway. If migration from surrounding soil occurs, it can cause ground settlement around the soakaway sufficient to affect the stability of adjacent buildings. The top surface of the granular fill should also be covered with geotextile to prevent the ingress of backfill material during and after surface reinstatement. Geotextile should not be wrapped around the outside of the ring units as it cannot be cleaned satisfactorily or removed when it has become blocked.

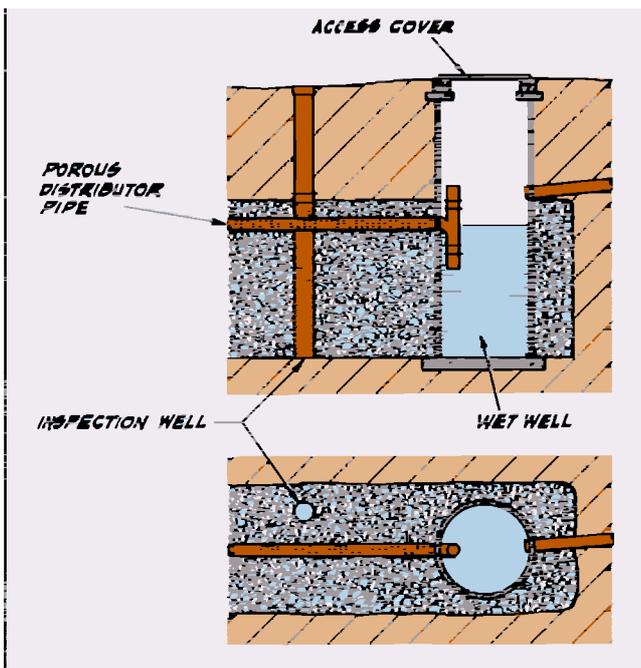


Figure 5 Trench-type soakaway with large wet well equipped with T-piece overflow to porous distributor pipe and separate inspection well

General considerations

Soakaways can provide a long-term, effective method of disposal of stormwater from impermeable areas of several hundreds of square metres. Long-term maintenance and inspection must be considered during the design and construction process. With wet well soakaways, vehicle-mounted suction emptying and jetting equipment can be used, so suitable access to inspection covers must be provided.

Pollution danger to the quality of groundwater must be considered. The limited evidence presently available suggests that roof surface run-off does not cause damage to groundwater quality and may be discharged directly to soakaways. Those pollutants entering the soakaway from roofs tend to remain in the soakaway, or in its immediate environs, attached to soil particles. However, paved surface run-off should be passed through a suitable form of oil interception device prior to discharge to soakaways. Maintenance of silt traps, gully pots and interceptors will improve the long-term performance, and the use of wet well chambers within the soakaway system can further assist in pollutant trapping and extending operating life.

Care must be taken so that the introduction of large volumes of surface run-off into the soil does not disrupt the existing sub-surface drainage patterns; it may be advantageous to use extended trench soakaway systems. The effect of ground slope must be considered when siting soakaways to avoid waterlogging of downhill areas.

Soakaways should not normally be constructed closer than 5 m to building foundations. In chalk, or other soil and fill material subject to modification or instability, the advice of a specialist geotechnologist should be sought as to the advisability and siting of a soakaway.

Site investigations must be undertaken thoroughly and competently so that all aspects of soil properties, geotechnology and hydrogeology are adequately reviewed alongside the hydraulic designs of soakaways.

Further reading

BS 8301: 1985 Code of practice for building drainage

Design and analysis of urban storm drainage.

The Wallingford Procedure.

Department of the Environment. National Water Council Standing Technical Committee Reports No 31. ISBN 0 901090 31 X.

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APPENDIX D

PHOTOGRAPHIC RECORD

Soakaway Testing Fieldwork

10 x A4



Plate 1—TP-S1



Plate 2—TP-S1



Plate 3—TP-S2



Plate 4—TP-S2



Plate 5—TP-S3



Plate 6—TP-S3



Plate 7—TP-S4



Plate 8—TP-S5



Plate 9—TP-S5



Plate 10—TP-S5A



Plate 11—TP-S5A



Plate 10—TP-S6



Plate 13—TP-S6



Plate 13—TP-S6



Plate 15—TP-S6



Plate 16—TP-S6



Plate 17—TP-S7



Plate 17—TP-S7



Plate 19—TP-S7



Plate 20—TP-S7